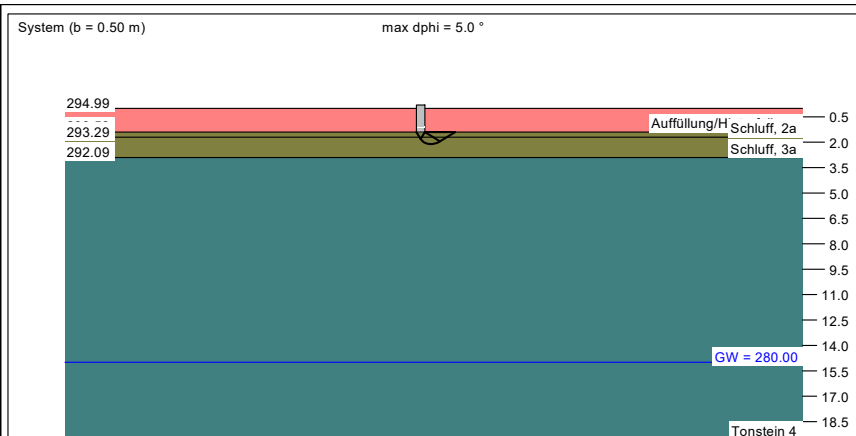




Boden	Tiefe [m]	γ [kN/m³]	γ' [kN/m³]	φ [°]	c [kN/m²]	E_s [MN/m²]	v [-]	Bezeichnung
	293.59	17.5	10.0	32.0	0.0	25.0	0.00	Auffüllung/Hinterfüllung
	293.29	18.5	9.5	26.0	3.0	10.0	0.00	Schluff, 2a
	292.09	19.0	10.0	27.0	2.0	10.0	0.00	Schluff, 3a
	<292.09	23.0	13.0	40.0	0.0	100.0	0.00	Tonstein 4

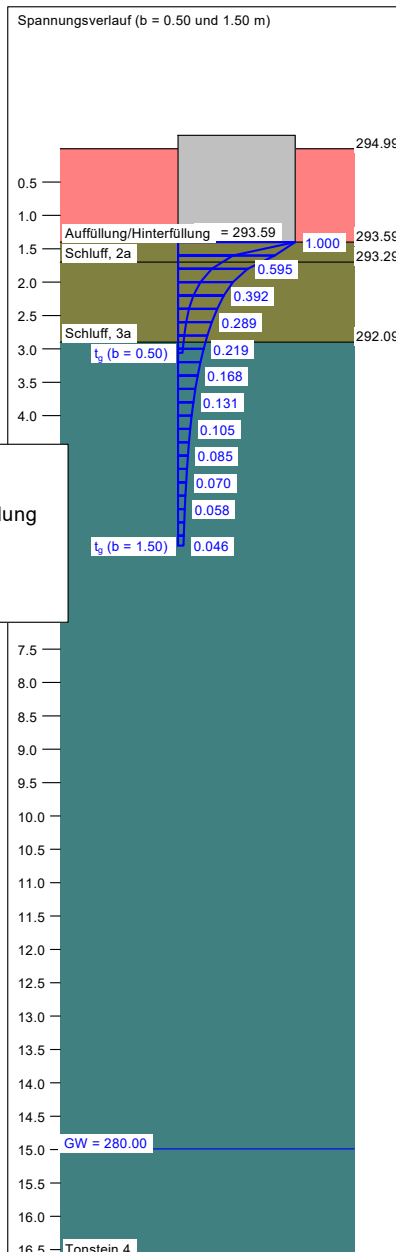
Oberkante Gelände = 294.99 m

a [m]	b [m]	$\sigma_{R,d}$ [kN/m²]	$R_{n,d}$ [kN]	$\sigma_{E,k}$ [kN/m²]	s [cm]	cal φ [°]	cal c [kN/m²]	γ_2 [kN/m³]	σ_0 [kN/m²]	t_g [m]	UK LS [m]	k_s [MN/m²]
0.50	0.50	408.5	102.1	286.6	1.01	26.7	2.33	18.72	24.50	3.06	2.11	28.3
0.55	0.55	412.0	124.6	289.1	1.10	26.7	2.30	18.74	24.50	3.18	2.18	26.2
0.60	0.60	415.4	149.5	291.5	1.19	26.7	2.27	18.76	24.50	3.31	2.25	24.5
0.65	0.65	418.7	176.9	293.8	1.28	26.7	2.25	18.77	24.50	3.43	2.32	23.0
0.70	0.70	422.0	206.8	296.1	1.36	26.8	2.23	18.79	24.50	3.54	2.40	21.7
0.75	0.75	425.2	239.2	298.4	1.44	26.8	2.22	18.80	24.50	3.66	2.47	20.7
0.80	0.80	428.4	274.2	300.6	1.52	26.8	2.20	18.81	24.50	3.77	2.54	19.7
0.85	0.85	431.6	311.8	302.8	1.60	26.8	2.19	18.82	24.50	3.88	2.61	18.9
0.90	0.90	434.7	352.1	305.1	1.68	26.8 *	2.18	18.83	24.50	3.99	2.68	18.1
0.95	0.95	437.8	395.1	307.2	1.76	26.8 *	2.17	18.84	24.50	4.09	2.75	17.5
1.00	1.00	440.9	440.9	309.4	1.83	26.8 *	2.16	18.85	24.50	4.20	2.82	16.9
1.05	1.05	471.3	519.6	330.7	2.02	27.4 *	1.95	18.87	24.50	4.37	2.93	16.3
1.10	1.10	541.5	655.2	380.0	2.40	28.6 *	1.63	18.99	24.50	4.64	3.07	15.8
1.15	1.15	595.7	787.8	418.1	2.72	29.4 *	1.48	19.13	24.50	4.87	3.19	15.4
1.20	1.20	654.3	942.2	459.2	3.07	30.1 *	1.36	19.28	24.50	5.10	3.31	15.0
1.25	1.25	718.7	1122.9	504.3	3.46	30.8 *	1.27	19.43	24.50	5.34	3.44	14.6
1.30	1.30	736.3	1244.4	516.7	3.63	31.0 *	1.21	19.53	24.50	5.49	3.53	14.2
1.35	1.35	739.1	1347.0	518.7	3.73	30.9 *	1.16	19.62	24.50	5.60	3.61	13.9
1.40	1.40	744.8	1459.8	522.7	3.84	30.9 *	1.12	19.70	24.50	5.72	3.70	13.6
1.45	1.45	747.9	1572.4	524.8	3.94	30.9 *	1.08	19.78	24.50	5.83	3.78	13.3
1.50	1.50	754.8	1698.3	529.7	4.05	30.9 *	1.04	19.87	24.50	5.95	3.86	13.1

* phi wegen 5° Bedingung abgemindert

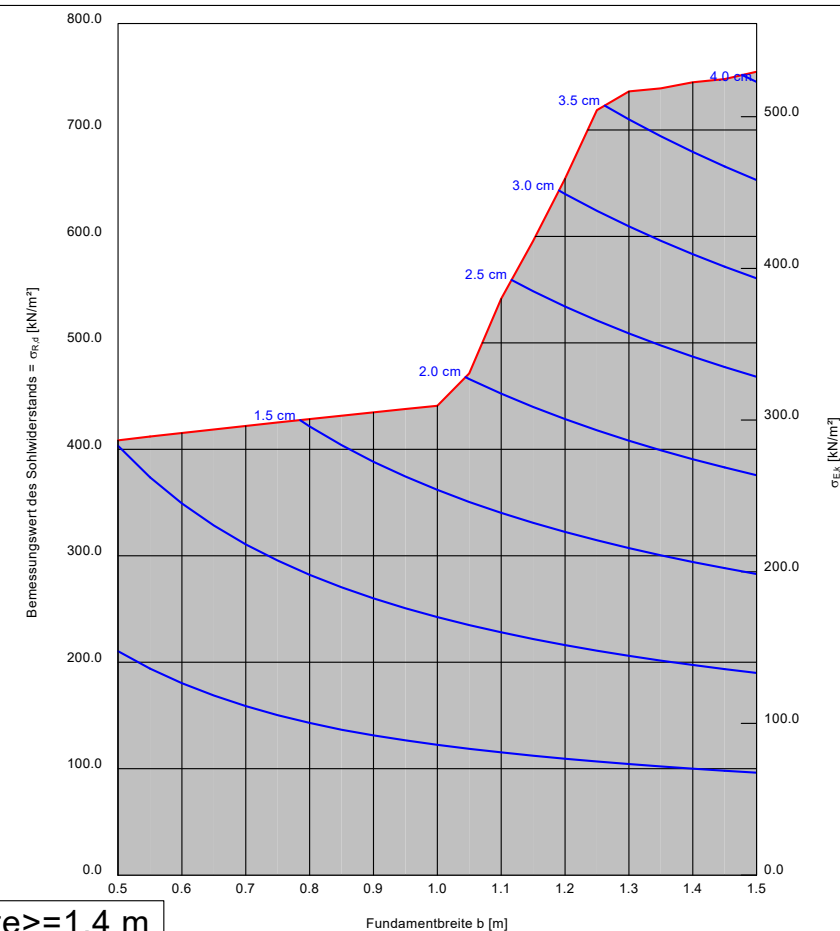
$\sigma_{E,k} = \sigma_{G,k} / (\gamma_{R,v} \cdot \gamma_{(G,Q)}) = \sigma_{G,k} / (1.40 \cdot 1.43) = \sigma_{G,k} / 1.99$ (für Setzungen)

Verhältnis Veränderliche(Q)/Gesamtlasten(G+Q) [-] = 0.50



Berechnungsgrundlagen:
Grundbruchformel nach DIN 4017:2006
Teilsicherheitskonzept (EC 7)
Einzelfundament (a/b = 1.00)
 $\gamma_{R,v} = 1.40$
 $\gamma_G = 1.35$
 $\gamma_Q = 1.50$
Anteil Veränderliche Lasten = 0.500
 $\gamma_{(G,Q)} = 0.500 \cdot \gamma_Q + (1 - 0.500) \cdot \gamma_G$
 $\gamma_{(G,Q)} = 1.425$
Oberkante Gelände = 294.99 m
Gründungssohle = 293.59 m
Grundwasser = 280.00 m

Grenztiefe mit p = 20.0 %
Grenztiefen spannungsvariabel bestimmt
Datei: 202385_AnI.9_ÜS km 98,909, te 1,4 m.gdg
— Solldruck
— Setzungen



te >= 1,4 m